

# DESIGN OF STEEL BEAMS...

## Exmples

A simply supported beam has an effective span of 7m and carries a uniformly distributed load of 50 kN/m (i.e DL = 25kN/m and LL = 25 kN/m). Taking  $f_y = 250 \text{ N/mm}^2$  and  $E = 2 \times 10^5 \text{ N/mm}^2$ , design the beam, if it is laterally supported.

## Example....

### STEP 1:

Factored Load =  $1.5 \times 50 = 75 \text{ kN/m}$  (Table 4)

### STEP 2:

Design Bending Moment =  $wl^2/8 = 459.375 \text{ kN.m}$

Design shear force =  $wl/2 = 262.5 \text{ kN}$

### • STEP 3:

Plastic Section modulus reqd.,  $Z_p = M \times \gamma_{mo} / f_y$   
(cl. 8.2.1.2)

$$= 459.375 \times 10^6 \times 1.1/250 \text{ (Table 5)}$$

$$= 2021.25 \times 10^3 \text{ mm}^3$$

## Example....

- $Z_p/Z_e$  is conservatively assumed to be 1.15

$$Z_{e, \text{ reqd}} = 2021.25 \times 10^3 / 1.15 = 1757.61 \times 10^3 \text{ mm}^3$$

**Choose ISMB 500**

Depth,  $h = 500 \text{ mm}$  ;

width of flange,  $b = 172 \text{ mm}$  ;

Thickness of flange,  $t_f = 17.2 \text{ mm}$  ;

Thickness of web,  $t_w = 10.2 \text{ mm}$  ;

Depth of web,  $d = h - 2(t_f + R) = 500 - 2(17.2 + 17) = 431.6 \text{ mm}$

$I_{zz} = 45218.3 \times 10^4 \text{ mm}^4$  ;       $Z_e = 1808.7 \times 10^3 \text{ mm}^3$

Weight of the section = 86.9 kg/m

### • STEP 4

Section Classification (Table 2)

$$\varepsilon = \sqrt{250/f_y} = 1$$

$$b/t_f = 172/17.2 = 10 < 10.5\varepsilon \rightarrow \text{compact}$$

$$d/t_w = 431.6/10.2 = 42.31 < 84\varepsilon \rightarrow \text{plastic}$$

Hence section is **compact**

- **STEP 5**

Check for adequacy of section

$$\begin{aligned} \text{Factored self weight} &= 1.5 \times 86.9 \times 9.81/1000 \\ &= 1.279 \text{ kN/m} \end{aligned}$$

$$\text{Total factored load} = 75 + 1.279 = 76.279 \text{ kN/m}$$

## Example....

- $M_{\max} = wl^2/8 = 467.21 \text{ kN.m}$

$$Z_p(\text{reqd.}) = 467.21 \times 10^6 \times 1.1/250$$

$$= 2055.72 \times 10^3 \text{ mm}^3 < 2080 \times 10^3 \text{ mm}^3$$

Hence provided section is adequate

- **STEP 6**

Design Shear Force ,  $V = wl/2$

$$= 76.279 \times 7/2 = 266.98 \text{ kN}$$

## Example....

### • STEP 7

$$\begin{aligned} \text{Design Shear Strength, } V_d &= V_n / \gamma_{mo} \\ &= h \times t_w \times f_{yw} / (1.1 \times \sqrt{3}) \\ &= 500 \times 10.2 \times 250 / (1.1 \times \sqrt{3}) \\ &= 669.201 \text{ kN} > 266.98 \text{ kN} \end{aligned}$$

**Hence OK**

**Also  $V < 0.6V_d$**

### • STEP 8

Check for Design Capacity

$$d/t_w = 42.31 < 67\epsilon \text{ (cl 8.2.1.1)}$$

$$\begin{aligned} M_d &= \beta_b Z_p \times f_y / \gamma_{mo} = 1 \times 2080 \times 10^3 \times 250 / 1.1 \\ &= 472.7273 \text{ kN.m} < 1.2 \times Z_e \times f_y / \gamma_{mo} \text{ (cl 8.2.1.2)} \\ &< 493.28 \text{ kN.m} \end{aligned}$$

**Hence satisfied**

### • STEP 9

Check for Deflection (Use unfactored imposed load)

$$\begin{aligned} \delta &= 5wl^4/384 = 8.64\text{mm} < l/300 \text{ (Table 6)} \\ &< 23.33\text{mm} \end{aligned}$$

**Hence safe**

- In the previous problem the bearing length was assumed to be adequate.
- Suppose a bearing length of 75mm is provided.
- We should check the safety of the web in bearing and buckling